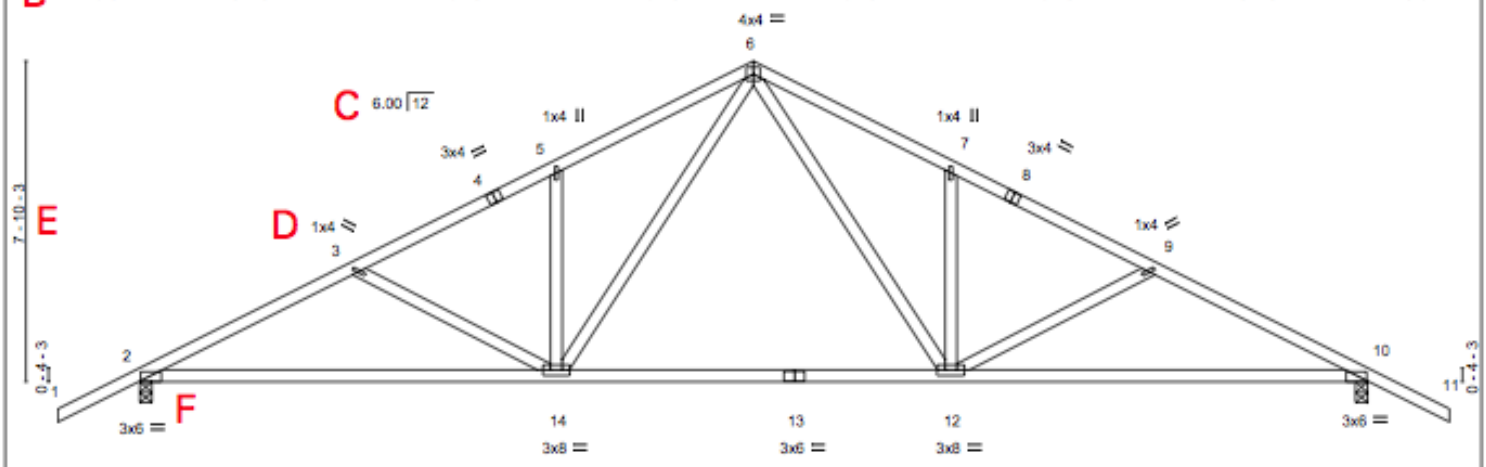
Job	Truss	Truss Type	Qty	Ply
DISPLAY	ROOF1	COMMON	1	1

A
-2-0-0 | 5-4-5 | 10-2-3 | 15-0-0 | 19-9-13 | 24-7-11 | 30-0-0 | 32-0-0
B
2-0-0 | 5-4-5 | 4-9-13 | 4-9-13 | 4-9-13 | 4-9-13 | 5-4-5 | 2-0-0



A
10-2-3 | 19-9-13 | 30-0-0
B
10-2-3 | 9-7-11 | 10-2-3
G

Plate Offsets (X,Y): [2:0-3-0,0-1-4], [10:0-3-0,0-1-4] H

LOADING (psf) I		SPACING 2-0-0 J		CSI M		DEFL N	in	(loc)	l/defl	PLATES P	GRIP
TCLL	20.0	Plates Increase	1.15	TC	0.29	Vert(LL)	-0.09	14	>999	MII20	249/190
TCDL	10.0	Lumber Increase	1.15 K	BC	0.83	Vert(TL)	-0.39	12-14	>907		
BCLL	0.0	Rep Stress Incr	YES	WB	0.36	Horz(TL)	0.07	10	n/a		
BCDL	10.0	Code	BOCA/ANSI95 L			1st LC LL Min l/defl = 240 O				Weight: 158 lb	

LUMBER Q
TOP CHORD 2 X 4 SYP No.2
BOT CHORD 2 X 4 SYP No.2
WEBS 2 X 4 SYP No.3

BRACING U
TOP CHORD Sheathed or 4-2-1 oc purlins.
BOT CHORD Rigid ceiling directly applied or 8-6-11 oc bracing.

R S
REACTIONS (lb/size) 2=1317/0-3-8, 10=1317/0-3-8
T Max Horz 2=-175(load case 5)
Max Uplift2=-341(load case 4), 10=-341(load case 5)

FORCES (lb) - First Load Case Only V
TOP CHORD 1-2=26, 2-3=-2024, 3-4=-1722, 4-5=-1722, 5-6=-1722, 6-7=-1722, 7-8=-1722, 8-9=-1722, 9-10=-2024, 10-11=26
BOT CHORD 2-14=1794, 13-14=1140, 12-13=1140, 10-12=1794
WEBS 5-14=-294, 7-12=-294, 3-14=-288, 6-14=742, 6-12=742, 9-12=-288

NOTES W
1) Unbalanced roof live loads have been considered for this design.
2) Wind: ASCE 7-98 per BOCA/ANSI95; 90mph; h=25ft; TCDL=5.0psf; BCDL=5.0psf; occupancy category II; exposure C; enclosed;MWFRS gable end zone; cantilever left and right exposed ; end vertical left and right exposed;Lumber DOL=1.33 plate grip DOL=1.33.
3) Provide mechanical connection (by others) of truss to bearing plate capable of withstanding 341 lb uplift at joint 2 and 341 lb uplift at joint 10.

LOAD CASE(S) Standard X

A Cumulative Dimensions
B Panel Length (feet - inches - sixteenths)
C Slope
D Plate Size and Orientation
E Overall Height
F Bearing Location
G Truss Span (feet - inches - sixteenths)
H Plate Offsets
I Design Loading (PSF)
J Spacing O.C. (feet - inches - sixteenths)
K Duration of Load for Plate and Lumber Design
L Code
M TC, BC, and Web Maximum Combined Stress Indices
N Deflections (inches) and Span to Deflection Ratio
O Input Span to Deflection Ratio
P MiTek Plate Allowables (PSI)
Q Lumber Requirements
R Reaction (pounds)
S Minimum Bearing Required (inches)
T Maximum Uplift and/or Horizontal Reaction if Applicable
U Required Member Bracing
V Member Axial Forces for Load Case 1
W Notes
X Additional Loads/Load Cases

Job	Truss	Truss Type	Qty	Ply	
DISPLAY	FLOOR1	FLOOR	1	1	Job Reference (optional)

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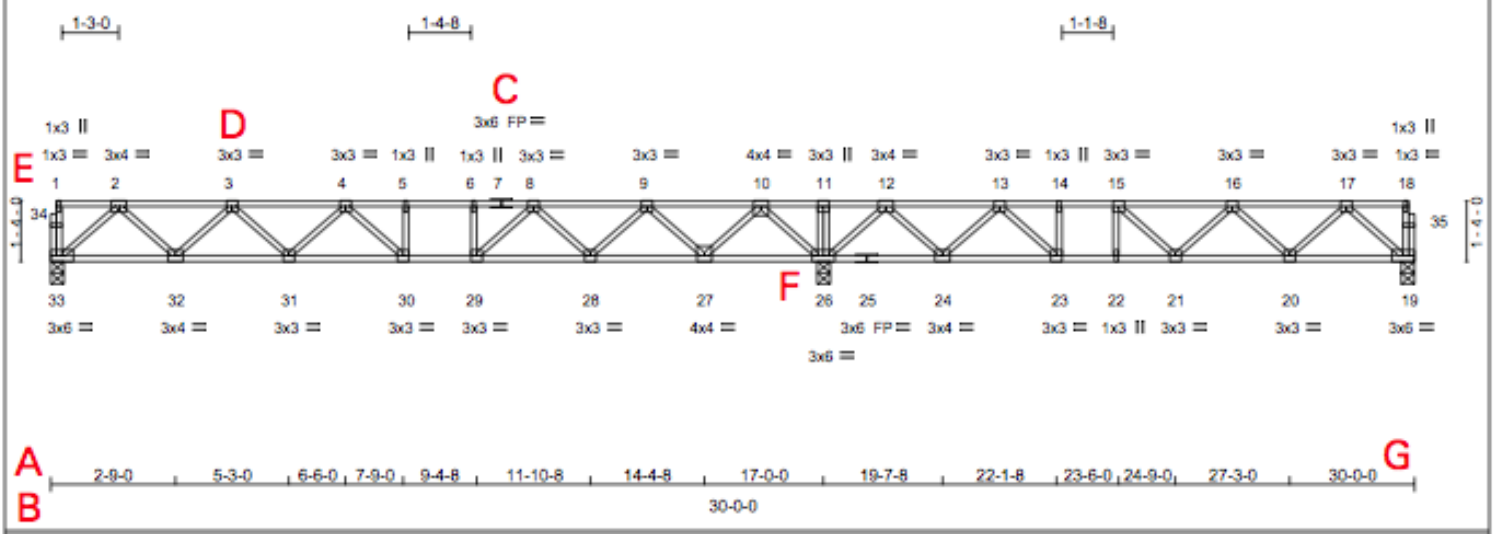

LOADING (psf) H		SPACING 2-0-0 I		CSI L		DEFL M	in	(loc)	l/defl	PLATES O	GRIP
TCLL	40.0	Plates Increase	1.00 J	TC	0.71	Vert(LL)	-0.18	30-31	>999	MII20	249/190
TCDL	10.0	Lumber Increase	1.00	BC	0.75	Vert(TL)	-0.24	30-31	>829		
BCLL	0.0	Rep Stress Incr	YES	WB	0.46	Horz(TL)	0.04	26	n/a		
BCDL	5.0	Code	BOCA/ANSI95 K	(Matrix)		1st LC LL Min l/defl = 360				Weight: 156 lb	

LUMBER P

TOP CHORD	4 X 2 SYP No.2
BOT CHORD	4 X 2 SYP No.2
WEBS	4 X 2 SYP No.3

BRACING T

TOP CHORD	Sheathed or 6-0-0 oc purlins, except end verticals.
BOT CHORD	Rigid ceiling directly applied or 6-0-0 oc bracing.

REACTIONS (lb/size) Q R 33=774/0-3-8, 19=508/0-3-8, 26=1978/0-3-8
Max Grav S 33=803(load case 2), 19=610(load case 3), 26=1978(load case 1)

FORCES (lb) - First Load Case Only U

TOP CHORD	33-34=-37, 1-34=-37, 19-35=-43, 18-35=-43, 1-2=-2, 2-3=-1353, 3-4=-2098, 4-5=-2186, 5-6=-2186, 6-7=-2186, 7-8=-2186, 8-9=-1264, 9-10=63, 10-11=2035, 11-12=2035, 12-13=467, 13-14=-732, 14-15=-732, 15-16=-951, 16-17=-784, 17-18=-2
BOT CHORD	32-33=826, 31-32=1859, 30-31=2282, 29-30=2186, 28-29=1785, 27-28=743, 26-27=-894, 25-26=-1091, 24-25=-1091, 23-24=183, 22-23=732, 21-22=732, 20-21=1032, 19-20=517
WEBS	11-26=-100, 2-33=-1097, 2-32=734, 3-32=-704, 3-31=332, 4-31=-256, 4-30=-130, 5-30=27, 10-26=-1519, 10-27=1157, 9-27=-1121, 9-28=725, 8-28=-724, 8-29=546, 6-29=-271, 12-26=-1257, 12-24=867, 13-24=-904, 13-23=747, 14-23=-299, 17-19=-685, 17-20=371, 16-20=-346, 16-21=-113, 15-21=298, 15-22=-234

NOTES V

1) Unbalanced floor live loads have been considered for this design.
2) Recommend 2x6 strongbacks, on edge, spaced at 10-0-0 oc and fastened to each truss with 3-16d nails. Strongbacks to be attached to walls at their outer ends or restrained by other means.
3) CAUTION, Do not erect truss backwards.

LOAD CASE(S) Standard W

A Cumulative Dimensions
B Panel Length (feet - inches - sixteenths)
C Chord Splice Face Plate
D Plate Size and Orientation
E Truss Depth
F Bearing Location
G Truss Span (feet - inches - sixteenths)
H Design Loading (PSF)
I Spacing O.C. (feet - inches - sixteenths)
J Duration of Load for Plate and Lumber Design
K Code
L TC, BC, Web Maximum Combined Stress Indices
M Deflections (inches) and Span to Deflection Ratio
N Input Span to Deflection Ratio
O MiTek Plate Allowables (PSI)
P Lumber Requirements
Q Reaction (pounds)
R Minimum Bearing Required (inches)
S Maximum Uplift and/or Horizontal Reaction if Applicable
T Required Member Bracing
U Member Axial Forces for Load Case 1
V Notes
W Additional Loads/Load Cases